

## ABSTRAK

### ANALISIS KESTABILAN LERENG BENDUNGAN URUG MENGUNAKAN *CROSSCORRELATION GROUND PENETRATING RADAR* TERHADAP *PIEZOCONE PENETRATION TEST* DI DAERAH SANGATTA PT.KALTIM PRIMA COAL

Muhammad Bakhtiar Risqa  
115150066

Pembangunan bendungan tipe urugan tanah membutuhkan nilai keamanan stabilitas lereng pada tubuh bendungan agar tidak terjadi longsoran. Analisis kestabilan lereng memerlukan data properti material yang diperoleh dari uji laboratorium sampel tanah yang berupa nilai berat isi dan sudut geser dalam. Salah satu parameter dalam properti material adalah nilai kohesi.

Nilai kohesi yang diperoleh dari sampel tanah melalui uji tekan bebas. Namun, nilai kohesi telah dipengaruhi oleh hasil uji pemboran tanah saat pengambilan sampel. Oleh karena itu perlu adanya nilai kohesi yang *natural* (tanpa terganggu) yang diperoleh dari tanah secara langsung menggunakan metode alternatif yaitu metode *Ground Penetrating Radar*. Analisis *crosscorrelation Ground Penetrating Radar* terhadap *Piezecone Penetration Test* memperoleh kohesi material dari konstanta dielektrik menggunakan Persamaan Robertson and Campanella (1985). Data Uji Laboratorium memperoleh sudut geser dalam dan bobot isi material.

Penelitian ini memperoleh persamaan empiris  $q_c = -0,74 \times \varepsilon_r + 3,25$  sehingga memperoleh material properti Tubuh bendungan urug memiliki litologi campuran tanah lempung, lanau dan pasir dengan kohesi  $14 \text{ kN/m}^2$ , bobot isi  $19 \text{ kN/m}^2$  dan tipe keruntuhan *Mohr Coloumb* dengan sudut geser dalam  $20^\circ$ . Perkuatan tanah menggunakan *Geocell*. Litologi Tanah dasar lempung lunak memiliki bobot isi  $20 \text{ kN/m}^2$  dan kohesi  $12 \text{ kN/m}^2$  dengan tipe keruntuhan *undrained*. Berdasarkan hasil Analisis Kestabilan Lereng pada Tubuh Bendungan menggunakan simulasi *software Slide 6.0* memperoleh nilai FK (*Safety of Factor*) 1,842 tergolong aman tanpa beban vertikal dan nilai FK 1,277 yang tergolong aman dengan beban maksimum sebesar  $18 \text{ kN/m}^2$ . Rekomendasi dari penelitian untuk melakukan *upgrading* material timbunan 1 meter pada tubuh bendungan urug di daerah Sangatta karena masih stabil dengan FK 1.282.

**Kata Kunci** : Konstanta Dielektrik,  $q_c$ , *Safety of Factor*

## ABSTRACT

### *SLOPE STABILITY ANALYSIS OF EMBANKMENT DAM USING CROSSCORRELATION GROUND PENETRATING RADAR TO PIEZOCONE PENETRATION TEST IN SANGATTA AREA PT.KALTIM PRIMA COAL*

**Muhammad Bakhtiar Risqa**  
**115150066**

The construction of earthfil-type embankment dams requires the safety value of the dam's body to prevent landslides. Analysis of slope stability is obtained material property data obtained from laboratory tests of soil samples that contain values of unit weight and internal friction angle. One parameter in material property is the value of cohesion.

Cohesion values obtained from soil samples through free press test. However, the cohesion value was agreed by the results of the drilling test when sampling. Therefore it is necessary to have a natural cohesion value (without being disturbed) obtained from the soil which directly uses an alternative method namely the Ground Penetrating Radar method. Analysis of the cross-correlating Ground Penetrating Radar to the Piezecone Penetration Test obtained cohesion material from the dielectric constant using the Robertson and Campanella Equations (1985). Laboratory Test Data obtain an internal friction angle and unit weight of material.

This study obtained the results of empirical calculations  $q_c = -0,74 \times \epsilon_r + 3,25$  so that the material obtained property Dam body requires a mixture of clay lithology, silt and sand with cohesion of  $14 \text{ kN/m}^2$ , unit weight of  $19 \text{ kN/m}^2$  and collapse type Mohr Coloumb with an internal friction angle  $20^\circ$ . Strengthening the ground using Geocell. Lithology Soft clay soils have a unit weight of  $20 \text{ kN/m}^2$  and cohesion of  $12 \text{ kN/m}^2$  with undrained collapse type. Based on the results of Slope Stability Analysis on the Dam Body using Slide 6.0 simulation software. Safety Factor value of 1,842 is classified as safe without vertical load and the FK value of 1,277 is classified as safe with a maximum load of  $18 \text{ kN/m}^2$ . Recommendations for research to increase 1 meter of pile material in the reservoir body in the Sangatta area because it is still stable with FK 1,282.

**Keyword** : Relative Permittivity,  $q_c$ , Safety of Factor